The Little Book of Tire Pavement Friction

Version 1.0 Submitted for Review and Comment

Pavement Surface Properties Consortium

September 2012

Gerardo W. Flintsch Kevin K. McGhee Edgar de León Izeppi Shahriar Najafi

Table of Contents

Table of Contents			
1. I	Introduction		
2. V	2. What Is the Effect of Tire Pavement Friction on Roadway Safety?		
3. V	3. What Is Tire Pavement Friction and Surface Texture?		
3.1	1. Basic Concepts of Friction	4	
3.2	2. What Is Pavement Texture?	4	
3.3	3. Components of Tire Pavement Friction	6	
3.4	4. Braking, Accelerating, and Cornering	7	
4. H	4. How Do We Measure Friction?		
4.1	1. Principles of Friction Measuring Equipment	10	
4.2	2. Macrotexture Measuring Techniques	11	
4.3	3. The effect of Hydroplaning		
4.4	4. What Operational Factors Affect Friction Measurements?		
4.5	5. Non-contact measurement of friction	15	
4.6	5. Dry friction	15	
5. V	What Is Equipment Calibration and Harmonization?	16	
5.1	1. Calibration	16	
5.2	2. Harmonization	16	
5.3	3. The International Friction Index	17	
6. V	What is a Pavement Friction Management Program?	17	
6.1	I. Friction demand		
6.2	2. Pavement Friction Management Programs (PFMP)		
6.3	3. Frequency of friction testing	19	
7. F	How do we Achieve and Maintain Adequate Tire pavement Friction	19	
7.1	1. Designing for Friction	19	
7.2	2. Restoring Friction	20	
8. Summary and Conclusions			
9. F	9. References		

List of Figures

Figure 1 Force body diagram for rotating wheel	4
Figure 2 Influence of texture wavelength on tire pavement interaction	
Figure 3 Texture Three Zone Concept of a wet surface	6
Figure 4 Key components of tire pavement friction	6
Figure 5 Friction versus slip	7
Figure 6 Force-body diagram for a wheel traveling around a curve with constant speed	9
Figure 7 Longitudinal Force Coefficient (LFC) Friction Testers	10
Figure 8 Circular Track Meter (CTMeter)	11
Figure 9 Effect of water film thickness on skid measurements	13

1. Introduction

Frictional properties of pavements play a significant role in road safety as the friction between tire and pavement is a critical contributing factor in reducing potential crashes. When a tire is free rolling in a straight line, the tire contact patch is instantaneously stationary and there is little or no friction developed at the tire/road interface, although there may be some interactions that contribute to rolling resistance. However, when a driver begins to execute a maneuver that involves a change of speed or direction, forces develop at the interface in response to acceleration, braking, or steering that cause a reaction between the tire and the road (called friction) which enables the vehicle to speed up, slow down, or track around a curve. To reduce the number of fatalities, injuries, and properties damage due to car crashes, the Federal Highway Administration (FHWA) issues guidance to highway agencies in management of pavement surface friction on roadways (FHWA 2010).

Car crashes can be due to several factors related with the driver, the vehicle, the environment, and the roadway infrastructure. Because the lack of sufficient friction between the tire and pavement during wet weather condition is one of the factors that can increase the risk of car crashes, it is important for Departments of Transportation (DOTs) to monitor the friction of their pavement networks frequently and systematically. This document provides guidelines for state DOTs and highway agencies to effectively use tire pavement friction data to support asset management decisions. The principles of friction and texture are explained in this document. Methods for measuring pavement surface friction and texture, and the factors that can affect their measurement, are further discussed. The importance of friction in safety design of highways is also highlighted.

2. What Is the Effect of Tire Pavement Friction on Roadway Safety?

Pavement Friction is very important to roadway safety because each year many people around the world lose their lives in vehicle-related incidents. In the United States it is one of the leading causes of death. According to the National Highway Traffic Safety Administration (NHTSA), in 2010, more than 2.2 million people were injured in the U.S. in car crashes, and, on average, there was a vehicle-related fatality every sixteen minutes. NHTSA also estimated that the economic cost of traffic crashes (2000) was \$230.6 Billion (NHTSA, 2010).

It has been shown that friction between tire and pavement is a critical factor in reducing crashes (Hall et al. 2009; Henry 2000; Ivey et al. 1992). Most of the skidding problems occur when the road surface is wet due to friction deficiencies. A study made in Kentucky in 1972 revealed that the rate of wet crashes increases as the surface friction drops below a certain value. Data for this study was collected on rural interstates and parkways. The study also confirmed the relationship between the rate of wet to dry crashes and pavement friction (Rizenbergs et. al., 1972).

In a study performed in Texas, it was found that higher percentage of crashes occur on roads with low friction while fewer crashes happen on roads with high friction (Hall et al. 2009). Many researchers have developed models to predict the association between car crashes and friction. Most studies confirm the association between high rates of car crashes and low levels of pavement friction.

3. What Is Tire Pavement Friction and Surface Texture?

3.1. Basic Concepts of Friction

According to the AASHTO Guide for Pavement Friction; "pavement friction is the force that resists the relative motion between a vehicle tire and a pavement surface" (Hall et. al, 2009). The friction force between tire and pavement is generally characterized by a dimensionless coefficient known as coefficient of friction (μ), which is the ratio of the tangential force at the contact interface to the longitudinal force on the wheel. These forces are shown in Figure 1.

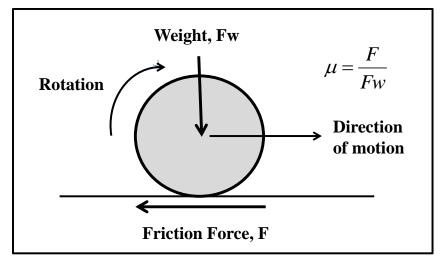


Figure 1 Force body diagram for rotating wheel

Tire pavement friction is the result of the interaction between the tire and the pavement, not a property of the tire or the road surface individually. This interaction plays a critical role in highway safety as it keeps the vehicles on the road by allowing drivers to make safe maneuvers. It is also used in highway geometric design to determine the adequate minimum stopping distance (Hall et al. 2009).

Although poor skid resistance is seldom the first cause of a crash – there is typically a human error that makes an emergency maneuver necessary – a crash will only occur if the friction demanded by the individual driver for the maneuver being attempted is greater than that which the road surface in that location (whether wet or dry), and the tires on the particular vehicle acting together can provide, in the particular set of circumstances at the particular time, and if skidding or wheel slipping leads to a loss of control or to a collision.

3.2. What Is Pavement Texture?

Pavement texture is defined by the AASHTO Guide for Pavement Friction as "the deviations of the pavement surface from a true planar surface" (Hall et al. 2009). To classify the characteristics of these deviations and their impact on pavement surface performance, the Permanent International Association of Road Congress (PIARC) has defined a scale based on the wavelength of the deviations (Figure 2).

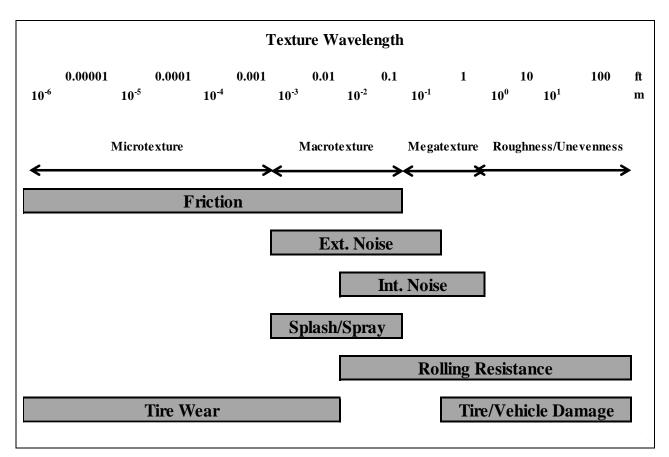


Figure 2 Influence of texture wavelength on tire pavement interaction (after Henry, 2000)

Tire pavement friction is dominated by the texture, or roughness, of the surface, with different texture components making different contributions. Of fundamental importance on both wet and dry roads is the *microtexture*, that is, the fine-scale texture (below about 0.5 mm) on the surface of the coarse aggregate in asphalt or the sand in cement concrete that interacts directly with the tire rubber on a molecular scale and provide adhesion. This component of the texture is especially important at low speeds but needs to be present at any speed.

On wet pavements, as speed increases skid resistance decreases and the extent to which this occurs depends on the *macrotexture*, typically formed by shape and size of the aggregate particles in the surface or by grooves cut into some surfaces. Generally, surfaces with greater macrotexture have better friction at high speeds for the same low-speed friction (Roe and Sinhal 1998) but this is not always the case¹. Since all friction test methods can be insensitive to microtexture under specific circumstances, it is recommended that friction testing be complemented by macrotexture measurement (ASTM E1845). It has been found that at speeds above 56 mph on wet pavements, macrotexture is responsible for a large portion of the friction, regardless of the slip speed (Hall et al. 2009).

¹ Recent research in the UK (Roe et al, 2008 and Roe and Dunford 2011, in preparation) has found that on certain types of modern asphalt (thin surface course layers with small coarse aggregate sizes), which have relatively low texture depth, lockedwheel friction decreases with speed but not to the extent expected from other HMA (or PCC) surfaces with comparably low macrotexture.

To better visualize the role of texture with the contact region of a tire on a wet pavement the Three Zone Concept, first suggested by Gough and later extended by Moore, is shown in Figure 3 (Moore, 1966). In zone 1, water is squeezed out by the macrotexture of the pavement surface, whereas in zone 2, the microtexture is responsible. Finally, in zone 3, the tire comes into dry contact with the pavement's surface. It is in this last zone, that the forces of adhesion and hysteresis come into play, as explained next.

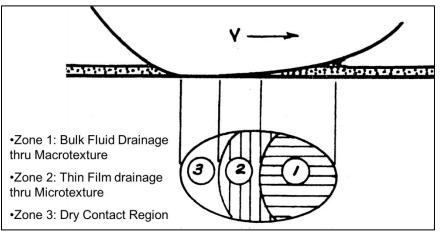


Figure 3 Texture Three Zone Concept of a wet surface (after Moore, 1966)

3.3. Components of Tire Pavement Friction

Tire pavement friction is the result of two main forces, adhesion and hysteresis. Adhesion is due to the molecular bonding between the tire and the pavement surface while hysteresis is the result of energy loss due to tire deformation. As the tire comes into contact with the pavement, the surface texture causes deformation in the tire rubber. This deformation is the potential energy stored in the tire. As the tire relaxes part of this energy will be recovered and part of it will be dissipated in form of heat. The generated heat (energy loss) is known as hysteresis. Both hysteresis and adhesion are related to surface characteristics and tire properties (Hall et al. 2009). These two key components of tire pavement friction are illustrated in Figure 4.

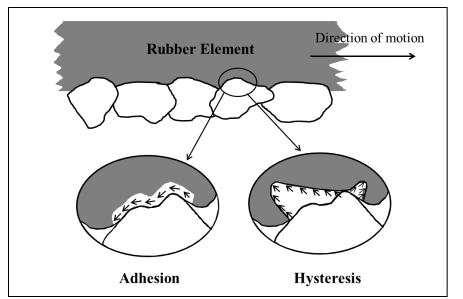


Figure 4 Key components of tire pavement friction (after Hall et al. 2009)

3.4. Braking, Accelerating, and Cornering

When a tire is free rolling in a straight line, the tire contact patch is instantaneously stationary and there is little or no friction developed at the tire/road interface, although there may be some interactions that contribute to rolling resistance. However, when a driver begins to execute a maneuver that involves a change of speed or direction, forces develop at the interface in response to acceleration, braking, or steering that cause a reaction between the tire and the road which enables the vehicle to speed up, slow down, or track around a curve.

During braking, as the braking force increases, the reacting force increases until it approaches a point at which the peak coefficient of friction available between the tire and the road is exceeded (this normally occurs between 18 and 30 percent slip). At this point (commonly known as "peak friction"), the tire continues to slow down relative to the vehicle speed and to slip over the road surface, even though the wheel is still rotating. If the braking force continues, the tire slips even more. Eventually complete locking of the wheel occurs, at which time the wheel stops rotating and the tire contact patch skids over the road surface. This is illustrated in Figure 5 below.

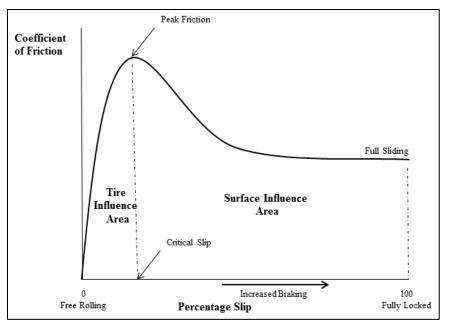


Figure 5 Friction versus slip (after Henry, 2000).

On a dry road surface, there is often little difference between peak and sliding friction and relatively little effect of speed. However, on a wet road, peak friction is often lower than in dry conditions, the sliding friction is typically lower than peak friction, and both usually (but not always) decrease with increasing speed. The differences between wet and dry, and peak and sliding friction, depend not only on vehicle speed and tire properties (including tread depth and pattern), but also to a large extent on the characteristics of the road surface, particularly its state of microtexture, the form and magnitude of the macrotexture, and the amount of water and other contaminants on the pavement (the importance of which is discussed further below). It is important to point out that when friction measurements occur on the left side of the peak, these will be mostly influenced by the characteristics of the tire, whereas those measurements made on the right side of the peak, will be influenced by those properties of the surface (macrotexture).

An analogous situation occurs during acceleration: although in normal circumstances the tire contact patch remains instantaneously stationary, too great a demand for acceleration can overcome the peak friction available and the wheel will start to slip, or in the extreme, to spin with little or no vehicle acceleration (as on ice).

Similarly, in cornering, side forces are generated that make the vehicle follow a curved path. If the combination of forward speed and the effective radius of curvature (influenced by the geometry of the road and steering angle) result in a demand for friction that exceeds what the road can provide, the wheel may slip sideways. If the demand is high enough to overcome peak friction, the wheel may slide sideways causing the vehicle to yaw. In this situation, a marked difference between peak and sliding friction could lead to a rapid loss of control.

The situation is exacerbated when braking and cornering occur simultaneously, because the available friction has to be shared between the two mechanisms. If the peak is exceeded, the sideforce goes down to near zero and the operator loses all control of steering. This is why anti-lock braking systems (ABS) are important. They detect the onset of wheel slip and momentarily release and then re-apply the brakes to make sure the peak friction is not exceeded and to reduce

the likelihood of side-slip occurring, thus helping the driver to maintain control. Similar ideas are used in some modern vehicle control systems to reduce the risk of side-slip occurring under simultaneous acceleration and cornering.

However, it is important to appreciate that while the instantaneous deceleration rates (and inversely stopping distances) with ABS functioning may be greater than for a vehicle skidding with locked wheels, there can be situations (particularly when the road is wet and the friction level is low) when the average friction (including the times when the wheel is released as well as those when it is slipping) will be less than in the locked-wheel condition.

4. How Do We Measure Friction?

Since friction depends on the interaction between the tire and the pavement, different measurements are obtained for different testing conditions. This has led to the development of different testing devices which operate under different conditions. As the tire freely roles on the pavement surface, longitudinal frictional forces generate at the tire and the pavement interface. The relative speed between the tire circumference and the pavement surface (slip speed) is zero (or very low) during free rolling (no braking) condition. Applying a constant brake to the tire will increase the slip speed to the potential maximum equivalent of the vehicle speed. This relationship can be mathematically expressed as follows (Hall et al. 2009):

$$S = V - V_P = V - (0.68 \times \omega \times r) \tag{1}$$

Where: S = Slip speed (mph)

V = Vehicle directional speed (mph) $V_p =$ Average peripheral speed of tire (mph) $\Omega =$ Angular velocity of the tire (radians/Second) r = Average radius of the tire (ft.)

If the average peripheral speed of tire (V_p) is equal to the vehicle speed, the slip speed (S) will be zero. During the fully locked wheel braking condition V_p is zero. This makes the slip speed to be equal to vehicle speed. Most literature refers to locked-wheel condition as 100 percent slip ratio and the free rolling condition as the zero slip ratio. The slip ratio can be mathematically expressed as follow (Hall et al. 2009):

$$SR = \frac{V - V_P}{V} \times 100 = \frac{S}{V} \times 100$$
Where: SR = slip ratio. (2)

When the vehicle steers around a curve or changes lanes another type of friction force generates at the tire pavement interface. This type of friction is called lateral (side-force) friction (Hall et al. 2009; Shahin 2005). The angle between the wheel and direction of travel is known as the "yaw angle". The force-body diagram of a vehicle steering on a curve is shown in Figure 6.

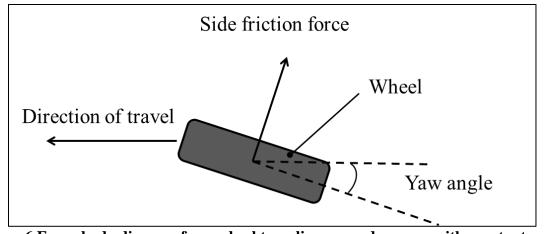


Figure 6 Force-body diagram for a wheel traveling around a curve with constant speed (after Hall, 2009)

According to this diagram, the side force friction can be defined as (Shahin, 2005):

$$SFC = \frac{sideways \cdot force}{vertical \cdot reaction \cdot between \cdot tire \cdot and \cdot road \cdot surface}$$
(3)
Where: SFC = sideways friction coefficient.

The slip ratio of the test wheel is related to the yaw angle of the wheel and it can be calculated as follows:

 $Slip \cdot ratio = Sin(\alpha)$ (4)
Where: $\alpha = Yaw$ angle.

Similar to slip speed, the slip ratio is zero during free rolling and it is maximum (100 percent) during fully locked condition.

4.1. Principles of Friction Measuring Equipment

A great many different devices have been developed over the years. They all rely on the broad principle of sliding rubber over a road surface and measuring the reaction forces developed in some way. There are essentially four general principles.

- i. Sliders, attached either to the foot of a pendulum arm or to a rotating head, which slow down on contact with the road surface. The rate of deceleration is used to derive a value representing the skid resistance of the road. A variant of this approach, still used by police forces in some parts of the world, is to measure the reaction force when a sled (with sliders representing car tires) is dragged over the road surface.
- ii. Longitudinal Friction Coefficient (LFC) measurement uses an instrumented measuring wheel mounted in line with the direction of travel. A fixed gear, or braking system, forces the test wheel to rotate more slowly than the forward speed of the vehicle. Consequently, the tire contact patch slips over the road surface and a frictional force is developed that can be measured. Typically, the ratio of vertical and drag forces is calculated (averaged over a fixed measuring length) to provide a value representing the LFC that is recorded. Within this category there is a wide range of slip ratios that may be used by individual devices. The slip ratio is usually governed by the control system to a fixed proportion of the forward speed which, in turn, determines the slip speed. Figure 7a

shows an example of a fixed-slip friction tester. Locked wheels (Figure 7b) measure the longitudinal friction by completely locking the brake of the measuring wheel, regardless of the test vehicle speed. Locked wheels can either use ribbed tire or smooth tire. Ribbed tires are knows to be less sensitive to pavement macrotexture and water film depth than smooth tires.

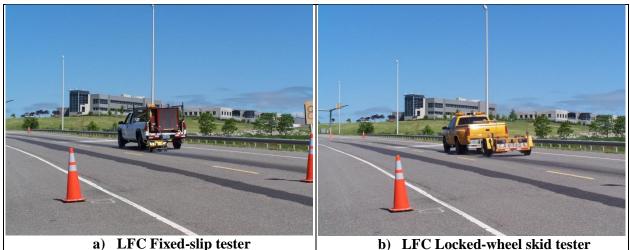


Figure 7 Longitudinal Force Coefficient (LFC) Friction Testers

- iii. Sideway Force Coefficient (SFC) measurement uses an instrumented measuring wheel set at an angle to the direction of travel of the vehicle. Although normally freely rotating, because it is set at an angle, the tire is made to slip over the road surface and the resulting force along the wheel axle (the "sideways force") is measured. The ratio of vertical and side forces averaged over a defined measuring length provides the value that is recorded to represent skid resistance. The wheel angle determines the slip ratio and the vehicle speed determines the slip speed.
- iv. Decelerometers are typically custom-made units mounted in a test vehicle, used to measure the deceleration of a vehicle under emergency braking. Widely used by police forces to assess road surface friction for collision investigations, and more recently in experimental naturalistic driving studies, these devices are not suitable for road network assessment or quality control purposes. They are mentioned here for completeness but are not considered further.

More agencies around the world have started using Continuous Friction Measuring Equipment (CFME) for highway friction management. CFMEs have the advantage that they continuously measure the friction across the entire stretch of a road, providing greater detail about spatial variability of the tire pavement frictional properties, using either SFC or LFC principles.

4.2. Macrotexture Measuring Techniques

Macrotexture can be measured using both highway speed profilers and static methods. While static devices can be used for project level measurements, the high speed devices are more appropriate for network level data collection. For static measurements, there are two basic techniques. First, the oldest one of these is the volumetric "patch" test. This is a test in which a known volume of sand or glass beads (or grease) is placed on the road surface and spread evenly

into a circular patch, filling the voids. The area is measured and thus the average depth below the peaks in the surface is calculated to give a value known as Mean Texture Depth (MTD).

In more recent years, laser displacement sensors, which measure along a narrow line traversed by the laser (rather than across the area of a patch of sand or glass beads), have been used to determine a surface profile from which a number of different parameters may be calculated to represent the texture depth. The most widely used parameter internationally, and defined in the ASTM E1845 standard, is the Mean Profile Depth (MPD), which attempts to estimate the average depth below the peaks in a 100-mm segment of the surface profile. The root mean square (RMS) texture depth has also been used extensively both in research and as part of friction management.

Numerous devices are now available to measure the MPD, such as the Circular Track Meter (Figure 8) for static measurements, and those attached to high-speed profilers (+64 kHz) that are primarily used to collect profile data for smoothness purposes in network evaluations. Static texture measuring device have a displacement sensor mounted on an arm which rotates at a fixed elevation from the surface collecting a high-resolution profile. Both, the high-speed and the static devices, are controlled by a computer that records the data and reports the processed data as MPD and RMS.



Figure 8 Circular Track Meter (CTMeter)

Research is showing that although both are useful in some situations, an alternative to RMS or MPD measures will likely be necessary to fully characterize road surface behavior. What that index should be is not clear at the moment. What is clear, is that a device that measures both, friction and macrotexture concurrently, is needed to determine both low-speed and high-speed friction performance (and their relative relevance in different situations) from a single measurement pass.

4.3. The effect of Hydroplaning

On dry pavement with good texture, the tire envelopes the texture and the depth of the penetration in the footprint is a tire property; however, when wet, water can partially prevent the

penetration if the macrotexture is not deep enough. Tire tread and macrotexture sometimes are not enough to allow the water to escape and thus causes the water to be present in the tire pavement interface.

In discussion of wet skidding crashes, reference is sometimes made to an effect known as hydroplaning or aquaplaning. This effect occurs when the water film on the road surface is so thick that it cannot be adequately broken up or dispersed by the passage of the tire. A thick film builds up in front of the tire that gradually spreads underneath it and lifts the tire off the road surface entirely, with a consequent major loss of traction even though the wheel can still rotate.

The tread pattern on the tire helps to prevent this phenomenon from occurring and for this reason many enforcing agencies set minimum tread depth requirements for vehicle tires. Macrotexture on the road also contributes to preventing aquaplaning. In countries that combine macrotexture requirements with tire tread limits, the effect is rare, occurring where a defective geometric design allows a significant build-up of water across the carriageway rather than the combination of a smooth (or well-worn) tire running on a smooth wet road surface. AASHTO has recently developed an automated cross-slope and drainage tool that recognizes the effect that these might have increasing the likelihood of hydroplaning (FHWA, 2011).

Therefore, it is suggested that when implementing a Pavement Friction Management Programs (PFMP), it collects friction, macrotexture, cross-slope, longitudinal grade, and the radius of curvature in a one-pass operations. This will allow for a proper subdivision and identification of different types of road that are adequately characterized with different friction thresholds.

4.4. What Operational Factors Affect Friction Measurements?

There are several operational factors that can affect the friction measurement. Better understanding of these factors can help highway agencies to establish standard testing condition and approaches for correcting measurement taken under different conditions.

- i. As mentioned in the previous section, water film thickness is one of the factors that have been proven to affect the friction measurements. The water on the pavement surface decreases the tire pavement contact area which results in reduction in friction. This effect is known to be more noticeable in higher speeds (>40 mph) compared to lower speeds (Hall et al. 2009).
- ii. Worn tires are known to be more sensitive to water film thickness. Pavement macrotexture and tires threads can provide channels for water to escape through the tire pavement contact area which result in increasing the traction between tire and the pavement surface. The effect of water film thickness on locked wheel skid trailer measurements is illustrated in Figure 9 and it suggests that smooth tires are more sensitive to the changes of water film thickness. Due to the lower sensitivity of ribbed tires to operational test conditions and water film thickness, some recommend them as the preferred choice for friction measurements (Henry 2000). However, ribbed tires are less sensitive to the pavement macrotexture, so it is recommended that their measurements be accompanied by macrotexture measurements.

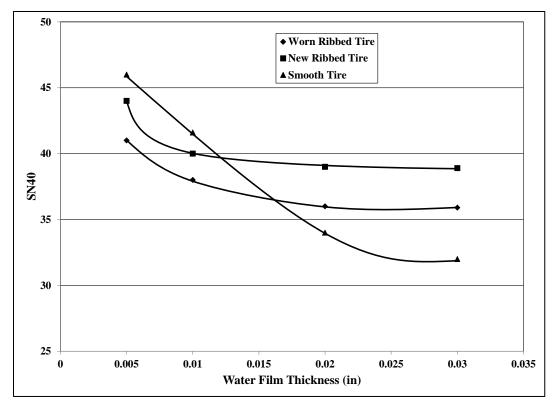


Figure 9 Effect of water film thickness on skid measurements (after Henry, 2000).

- iii. Recent studies have also confirmed the sensitivity of Continuous Friction Measuring Equipment (CFME) to water film thickness and other operational test conditions (Najafi et al. 2012).
- iv. Speed is also a factor. Standards for locked-wheel friction measurements (SN, skid numbers) are set at 40 mph. Present vehicle operating speeds in Interstate and Primary Highways are much higher than this, while urban areas often have speed limits below.
- v. Since both hot mix asphalt surface and tires are viscoelastic materials, temperature also affect their properties. Research has indicated that tire pavement friction decreases if the tire temperature increases (Hall et al. 2009). Although several researchers have investigated on this effect, this phenomenon is not still very well understood.
 - a. Jayawickrama and Thomas (1998) found that variation in skid numbers measurements can be as significant as 10 to 12 skid numbers from one day to another. These variations are due to changes in temperature and precipitation (Jayawickrama and Thomas 1998).
 - b. During summer months, skid resistance is lower than other times of the year. This effect can be either due to accumulation of polished particles from pavement which decrease the microtexture and macrotexture, or it can be contamination from vehicles such as oil and grease dripping. During the winter, winter maintenance practices, such as applying deicing salt, cause surface wear which expose new particle on aggregate surface and improves skid resistance. In spring, heavy rain flushes out the fine grit and makes the aggregate surface courser. The

course aggregate surface provides higher macrotexture and consequently higher skid resistance (Jayawickrama and Thomas 1998).

- c. Colony (1992) also reported that fluctuation of friction trough out the year has the highest values in the winter and the lowest friction is experienced at the end of the summer (Colony 1992). Faung and Hughes (2007) detected that skid measurement on SUPERPAVE mixes, follow a cyclic pattern with the higher values in winter and low values in fall and summer (Faung and Hughes 2007).
- d. Changes in temperature do not have a direct effect on the friction of pavement surface. However, they can affect the properties of the skid tester's tire (Jayawickrama and Thomas 1998). As explained before, tire pavement friction is composed of adhesion and hysteresis. Adhesion is the shear force generated at the interface of the contact area and hysteresis is due to the damping losses in the tire rubber (Li et al. 2004). Higher temperature makes the tire more flexible. This reduces the energy loss of the tire (hysteresis) and decreases the measured skid number. Nevertheless there is no proof available for this mechanism in the literature. While some studies stated that the effect of temperature is a very insignificant; many others indicate that temperature is a significant factor (Jayawickrama and Thomas 1998).
- e. Bazlamit and Reza (2005) indicated that regardless of the surface texture, increasing the temperature decreases the hysteresis component of surface friction while for adhesion component, surface texture affects this behavior (Bazlamit and Reza 2005). Since hysteresis accounts for greater part of total friction, the combined friction of the surface decreases with increasing temperature (Bazlamit and Reza 2005).
- f. Hill and Henry (1982) proposed a model that predicts the seasonal variation in the skid number intercept (SN_0) . The analysis was based on the data collected on test sites in Pennsylvania from 1978 to 1980.

4.5. Non-contact measurement of friction

Emerging technologies are evaluating the computation of the pavement friction based on data from the vehicle (deceleration, slipping information for the traction control systems, etc.) and/or non-contact technologies (e.g., laser-based systems or image-processing) to measure microtexture. These alternatives to rubber-on-road techniques are laudable ideas and presently there are plenty of researchers looking into this, but they are, at the moment, not reliable techniques and are unlikely to work at traffic speed for some time.

For the immediate and probably medium term, "dragging a tire across a road" remains the only reasonably reliable way to assess the skid resistance of a road. It does at least have the virtue of measuring something relating to the rubber and the road – even though it will need to be standardized so that the measurement is transportable and not uniquely biased towards one rubber-road interaction with limited relevance to all others.

4.6. Dry friction

Most work on skid resistance relates to wet roads. However, there may be situations in which dry road friction becomes a contributory factor in crashes. In the 1980s, Roger Hosking from TRL (then the TRRL) carried out a study investigating the effect of seasonal variations on

accident frequency in the U.K. (Hosking 1986). He observed that on most of the geographical regions and groups of roads studied, changes in dry-road skidding rates were related to wet-road skid resistance. However, he attributed this apparent correlation at least in part to misreporting of roads in the accident statistics as dry when they were actually damp.

Since then, in-service skid resistance standards have been enforced since 1987, and the vast majority of roads (especially the main highways) now have good wet skid resistance and macrotexture. It has been found (as do the police investigating crashes) that the measured dry friction (locked-wheel) is pretty consistently similar across the network and that wet friction is much more variable. Therefore, it is recommended that a PFMP focus on the worst case of wet road conditions.

5. What Is Equipment Calibration and Harmonization?

Calibration of equipment is generally done to make sure the equipment is performing properly and ensure that measurements are accurate. Although calibration can improve the accuracy of the equipment, there will still be some error in the measurements. It has been proposed that to minimize the effect of error in the accuracy of the equipment, measurements should be adjusted to a common scale. This process is called harmonization and is generally proposed after calibration.

5.1. Calibration

Equipment calibration involves all the periodic checks that are performed on the friction tester to ensure that the instrument is collecting accurate measurements. Most equipment users perform in-house calibration prior to testing. This includes calibration of water delivery system, speed measuring equipment, and force/torque transducers (Henry 2000).

ASTM standards E274 and E2793 provide instructions regarding to calibrating the speed and skid resistance force of the full-scale tire friction testers. Speed calibration can be done by measuring the travel time of the friction tester on a pre-measured length of pavement surface. The surface should be reasonably flat and straight and the test vehicle should be loaded to the normal test conditions. Force transducer calibration can also be performed in accordance with ASTM E274.

In the US, two specialized calibration centers, the Transportation Research Center Inc. (TRC Inc.) in Ohio and the Texas Transportation Institute (TTI) in Texas, provide E2793 Area Reference Friction Measurement Systems (ARFMS), along with several asphalt and concrete test surfaces that are used to calibrate locked-wheel friction testers based on ASTM standards.

5.2. Harmonization

ASTM has defined harmonization of measurements as "the adjustments of the outputs of different devices used for the measurement of a specific phenomenon so that all devices report the same value" (ASTM E 2100). Several studies dealing with harmonization of friction measurement equipment have been made. These include the Permanent International Association of Road Congress (PIARC or World Road Association) International Experiment from the early 1990s, the European HERMES project, the NASA Friction Workshops at Wallops Flight Facility, the Virginia Tech Transportation Institute (VTTI) Pavement Surface Properties

Consortium Rodeo Reports, and the "Tyre and Road Surface Optimisation for Skid resistance and Further Effects" (TYROSAFE) (Adewole 2008).

5.3. The International Friction Index

The PIARC experiment developed the International Friction Index (IFI) to compare and harmonize between various methods used around the world to measure friction and texture (Wambold et al. 1995). The IFI is composed of two parameters: a speed constant (Sp) and a friction number at 60 km/hr. (F60). The speed constant (Sp) is ideally predicted by a macrotexture measurement.

According to ASTM E1960, the IFI can be calculated following the following steps:

- i. Measure the friction FR(S) at the recommended slip speed (S) for the device used
- ii. Measure the Mean Profile Depth (MPD) based on ASTME1845
- iii. Calculate the speed constant (Sp) according to:

$$S_n = 14.2 + 89.7 MPD$$

Convert the friction at the measured slip speed S (FR(S)) to the friction at the standard 60 km/hr. friction (FR60):

$$FR60 = FRS \times e^{\frac{S-60}{S_p}}$$
(6)

where: FR60 = Adjusted friction FRS = Measured friction S = Slip speed

iv. The final step is to calculate the calibrated friction F60:

$$F60 = A + B \times FRS \times e^{\frac{S-60}{a+b \times TX}} + C \times TX$$
(7)

where A, B and C are the calibration factors for the friction equipment used, and a and b, are the ones for the texture equipment used.

The IFI model builds on an early model developed at Penn State that had the same shape but used a different parameter, the friction at zero speed, μ_0 , and the percent normalized gradient, PNG (Henry 2000).

6. How to implement Pavement Friction Management Programs

Pavement friction management includes both engineering practices to provide a pavement surface with adequate and durable friction and also periodic data collection and analysis to ensure the effectiveness of these practices. There will be occasional situations where a sudden change of friction on the road can cause crashes if it happens to be in the "wrong place" – perhaps just where a driver has chosen to apply the brakes or in the sharpest part of a curve where there has previously been sufficient traction for the radius and speed – but even then its causative potential and the severity of the result depend on the combinations of other circumstances. Such changes might be more important for certain vehicle types – motorcycles, for example.

(5)

If the reason that the road is slippery is due to some external factor, such as ice on the surface or a local oil spillage, there is little that the road engineer can do, apart from taking measures to prevent ice from forming or having procedures in place to clean up spills. If, however, the road has inherently poor skid resistance because of the materials of which it is made and how those materials have reacted to the passage of traffic over time, then it can be said that the road may contribute to crashes and road engineers should be able to detect such situations and take appropriate action. Inadequate cross slope and/or grade can also contribute to crashes by contributing to flooding of the road.

6.1. Friction demand

Factors such as traffic volume, geometrics (curves, grades, cross-slope, sight distance, etc.), potential for conflicting vehicle movements, and intersections should be considered for determining friction demand. Curves and intersections also tend to lose friction at a faster rate than other roadway locations and thus justify a higher friction demand. Thus, friction demand is the level of friction needed to safely perform braking, steering, and acceleration maneuvers. Highway agencies can establish Investigatory Level (desirable) and Intervention Level (minimum) values for pavement friction and texture in accordance with the AASHTO Guide for Pavement Friction.

6.2. The role of Pavement Friction Management Programs (PFMP)

The role of a Pavement Friction Management Programs (PFMP) or policy is to provide a framework by which road engineers can monitor the condition of their networks and, based on objective evidence, make appropriate judgments regarding treating or resurfacing the road in those situations that require it. This involves balancing the risk of a crash occurring with the costs and practicalities of providing adequate friction.

There will be significant sections of the network, especially lightly trafficked routes or on major highways with two or more lanes and traffic flowing in one direction only, where situations likely to involve skidding are generally rare or where the contributory factors are completely unpredictable (even those caused by braking due to traffic weaving). It could be argued that, in these circumstances, it is not practical to expect to provide extremely high performance on the off-chance that someone (whether prompted, for example, by a wild animal running on to the road or an unexpected action from another road user) decides to brake suddenly or swerve sharply on that particular spot. On the other hand, in places where it is known that drivers frequently need to brake or turn at speed, for instance, higher friction levels are likely to be needed than would be adequate elsewhere.

By enabling vehicles to reduce speeds more rapidly or allowing control to be retained for longer, the consequences of a crash in terms of death or severity of injury may be improved. Although crashes will probably never be completely eliminated, an effective policy can reduce collision risk and reduce the severity level of those crashes that do happen. Therefore, in developing a PFMP, all these factors will need to be considered. However, at the heart of the PFMP is the monitoring of skid resistance on the network with the appropriate measuring equipment.

Since the range of factors causing crashes is so broad, it is next to impossible to decide on a test method that can be directly predictive and cover all circumstances. In the long term, multi-

function devices might provide a solution, but it is unlikely that any one device can be specifically linked to crash prediction. Rather, measurements should be used to set criteria for judging the road based on an analysis of crashes, as it is recommended in the AASHTO Guide for Pavement Friction (Hall et al. 2009) and as is the approach in the other countries (Viner et al. 2005).

Through appropriate pavement design, construction, and maintenance practices; highway agencies can ensure their pavement surfaces provide adequate friction. FHWA Technical Advisory TA 5040.36 "Surface Texture for Asphalt and Concrete Pavements" provides guidelines on state-of-the-practice techniques for providing adequate surface texture and friction (FHWA 2010).

Locations with high rate of wet-weather crashes need to be identified and investigated for the purposes of minimizing friction-related crash rates. The procedure is commonly done by calculating the ratio between wet weather crashes and total crashes (wet + dry) and then following one of the subsequent approaches (FHWA 2010):

- i. Agencies may use a specific value for the wet crash ratio above which a location will be identified as an elevated wet-weather crash location. Depending on geographical and climate circumstances this ratio can vary between 0.25 and 0.5.
- ii. Agencies may compare the wet crash ratio with the average ratio for that functional class of highway in that area. If the computed ratio is above the average by a specified percentage that location is identified as an elevated wet-weather crash location.
- iii. A minimum number of wet-weather or total crashes within a segment is another criterion that some agencies use in order for a segment to be identified as an elevated wet-weather crash location.

6.3. Frequency of friction testing

Roads with the highest traffic volumes, the highest likelihood of changes in friction over time, and the highest friction demand justify the most frequent monitoring of friction (FHWA 2010). A risk-based approach can be implemented to determine the frequency of the friction test for roadway networks. Roads with higher friction demand require more frequent friction monitoring. Many agencies monitor the friction of their important roadway network on an annual basis, while a 2 to 5 year cycle may be appropriate for the part of network with lower-risk (FHWA 2010).

Network friction monitoring is generally not necessary in both wheel paths. The left wheel path is generally considered to have the most traffic due to passing maneuvers and is the most frequently tested in the US (FHWA 2010).

7. How do we Achieve and Maintain Adequate Tire pavement Friction

7.1. Designing for Friction

Pavement friction design involves utilizing proper materials and construction techniques to achieve high level of microtexture and microtexture in pavement surface. The type of aggregates used in the surface mix directly affects the microtexture while gradation and size of aggregates

governs the macrotexture properties of pavement surface (Hall et al. 2009). In asphalt mixtures, large aggregates govern the frictional properties of the surface while for concrete mixes, fine aggregates control the frictional properties (Hall et al. 2009).

The wear characteristics of aggregates are also important in maintaining proper friction level. Aggregates mineralogy and hardiness directly affect the durability and polish ability of the aggregates. It is generally better to have aggregates with different size and wear characteristics in the mix so they can constantly renew the surface (Hall et al. 2009).

7.2. Restoring Friction

There are several methods that can be used to restore the frictional properties of old pavements. Diamond grooving is a techniques that is used mostly in order to improve the frictional properties of the concrete pavement surfaces (Martinez 1977). The method uses diamond infused steel cutting blades for grinding and grooving concrete pavement. For grinding, the blades are spaced close together so that they can cut the pavement's unevenness (megatexture) and leave a rough pavement surface (high microtexture). For grooving, the blades are further spaced out so they create channels on the pavement surface which increases the friction by improving water drainage (high macrotexture) (Wulf et al. 2008).

Using High Friction Surfaces (HFS) in epoxy overlays is another option to restore surface friction, on both concrete and asphalt pavements. This method can increase the surface friction without jeopardizing other surface characteristics including noise and durability. HFS treatments utilize a durable aggregate and some type of resin (binder) to hold the aggregate particles together and glued to the road surface. There are several types of HFS commercially available in the market including: Cargill, Tyregrip, Italgrip, Crafco, Flexogrid, etc. (Roa 2008).

8. Summary and Conclusions

It is important for highway agencies to monitor the pavement friction periodically and systematically to support their safety management programs. The data collected data can help implement preservation policies that improve the safety of the roadway network and decrease the number of skidding-related crashes.

This document discusses principles of tire pavement friction and surface texture. Methods for measuring friction and texture are further discussed. The importance of friction in safety design of highways is also highlighted. The information in this document can be used by state DOTs and highway agencies to effectively use tire pavement friction data for supporting asset management decisions.

9. References

- 1. Adewole, A. (2008). "Deliverable 02: Report on Dissemination Strategy, Tyre and Road Surface Optimisation for Skid Resistance and Further Effects (TYROSAFE), Forum of European National Highway Research Laboratories (FEHRL)."
- 2. Bazlamit, S., and Reza, F. (2005). "Changes in Asphalt Friction Components and Adjustment Number for Temperature." The Journal of Transportation Engineering, ASCE 2005:131-470.
- Colony, D. (1992). "Influence of traffic, surface age and environment on skid number." Ohio Department of Transportation Project Number 14460 Final Report, Columbus, Ohio.
- 4. Faung, H., and Hughes, W. (2007). "Friction Monitoring of SuperPave Mixes in Virginia." Virginia Highway & Transportation Research Council: 8-9.
- 5. FHWA (2010). "Technical Advisory T5040.38, Pavement Friction Management."
- 6. Hall, J. W., Program, N. C. H. R., and Board, N. R. C. T. R. (2009). Guide for pavement friction, National Cooperative Highway Research Program, Transportation Research Board of the National Academies.
- 7. Henry, J. J. (2000). Evaluation of pavement friction characteristics, Transportation Research Board.
- 8. Henry, J. J. (2000). "Evaluation of Pavement Friction Characteristics: A Synthesis of Highway Practice, NCHRP Synthesis 291, Transportation Research Board, National Research Council, Washington, D.C.".
- 9. Hosking, J. (1986). "Relationship between skidding resistance and accident frequency estimates based on seasonal variation." TRRL Research Report 76, Transport Research Laboratory, Crowthorne, UK.
- 10. Ivey, D. L., Griffin III, L. I., Lock, J. R., and Bullard, D. L. (1992). "Texas Skid Initiated Accident Reduction Program."
- 11. Jayawickrama, P., and Thomas, B. (1998). "Correction of Field Skid Measurements for Seasonal Variations in Texas." Transportation Research Board 1639:147-152.
- 12. Li, S., Noureldin, S., and Zhu, K. (2004). "Upgrading the INDOT Pavement Friction Testing Program." Joint Transportation Research Program, Perdue Libraries: 6-9.
- 13. Martinez, J. (1977). "Effects of pavement grooving on friction, braking, and vehicle control."
- 14. Najafi, S., Flintsch, G. W., and McGhee, K. K. (2012). "Assessment of operational characteristics of continuous friction measuring equipment (CFME)."
- 15. Roa, J. A. (2008). "EVALUATION OF INTERNATIONAL FRICTION INDEX AND HIGHFRICTION SURFACES." Virginia Polytechnic Institute and State University.
- 16. Roe, P. G., and Sinhal, R. (1998). "The Polished Stone Value of aggregates and inservice skidding resistance." TRL Report 322, TRL, Crowthorne, UK.
- 17. Shahin, M. Y. (2005). Pavement management for airports, roads, and parking lots, Springer Verlag.
- Viner, H. E., Parry, A. R., and Sinhal, R. (2005). "Linking road traffic accidents with skid resistance – recent UK developments, International Conference on Surface Friction of Roads and Runways, Christchurch, New Zealand."
- 19. Wambold, J., Antle, C., Henry, J., and Rado, Z. (1995). "PIARC (Permanent International Association of Road Congress) Report." International PIARC Experiment to

Compare and Harmonize Texture and Skid Resistance Measurement, C-1 PIARC Technical Committee on Surface Characteristics, France.

- 20. Wulf, T., Dare, T., and Bernhard, R. (2008). "The effect of grinding and grooving on the noise generation of Portland Cement Concrete pavement." Journal of the Acoustical Society of America, 123(5), 3390.
- 21. NHTSA, Traffic Safety Facts 2010, US DOT, (Accessed on September 12, 2012), [online] available at: <u>http://www-nrd.nhtsa.dot.gov/Pubs/811659.pdf</u>
- 22. Rizenbergs, R.L., Burchett, J.L., and Napier, C.T., 1972. "Skid Resistance of Pavements," Report No. KYHPR-64-24, Part II, Kentucky Department of Highways, Lexington, Kentucky.
- 23. Moore, D.F. (1966), Prediction of Skid-Resistant Gradient and Drainage Characteristics of Pavements, Highway Research Record 131, Highway Research Board, Washington, D.C., pp. 181-203.
- 24. FHWA, Focus, October 2011. (Accessed on September 12, 2012), [online] available at: http://www.fhwa.dot.gov/publications/focus/11oct/11oct.pdf